

BUILDING MATERIALS AND PRODUCTS

UDC 666.94.002.35

O. B. Kukina¹, S. S. Glazkov², D. E. Barabash³

ORGANO-MINERAL MODIFIER OF CLAY SOILS STRENGTHENED BY CEMENT

Voronezh State Technical University

Russia, Voronezh, tel.: (473)2717-617, e-mail: lgkkn@rambler.ru

¹*PhD in Engineering, Assoc. Prof. of the Dept. of Chemistry and Chemical Technology of Materials,*

²*D. Sc. in Engineering, Prof. of the Dept. of Chemistry and Chemical Technology of Materials*

Russian Air Force Military Educational and Scientific Center

«Air Force Academy Named after Professor N. Ye. Zhukovsky and Yu. A. Gagarin»

Russia, Voronezh, tel.: +7-908-146-94-66, e-mail: barabash60170@yandex.ru

³*D. Sc. in Engineering, Prof., Head of the 31-st Chair of Research and Design of Airfields*

Statement of the problem. The problem of deacceleration of cement stone setting in reinforced clay soils by means of a suggested complex organomineral modifier is presented.

Results. An organo-mineral additive for cement stone modification is synthesized. The dynamics of the structure formation of the additive particles in the process of synthesis is determined. The influence of the modifier on the strength of cement stone and soil cement is studied. The rational composition of the modified soil cement the strength of which exceeds the strength of the traditional soil cement by 1.5 times is specified.

Conclusion. The effectiveness of the use of a cement stone synthesized modifier as a soil stabilizer and deaccelerator of the cement laying rate of setting in the process of clay soils strengthening is proved. The prospects of the use of the designed modifier are presented, the most rational areas of its use, namely for soil bases structure and highway rural road pavement, are determined.

Keywords: organo-mineral modifier, clay soils, reinforcement of soils.

Introduction. Strengthening of clayous soils using modern modifiers has been very important following stipulated by the Address of the President of the Russian Federation to the Federal Assembly on December 12, 2012 on the double rate in the funds on construction and reconstruction of highways in 2013—2022 compared to the last decade. Soil modifiers are widely

used in construction and reconstruction of highways and runways of different types: logistic sites and shopping centers, port and customs terminals, parking lots and forest/park pathways, hydroinsulation of (toxic) waste dump sites [3, 5, 7, 13, 17, 19].

The use of organic and organic and mineral stabilizers in soil strengthening is impossible without analyzing structural and functional features of modifiers and cement soils [11, 20]. Reactive functional oligomers certainly have a leading position in structure formation of strengthened soils [11] as well as processing of secondary raw materials [4, 12, 15]. It is essential that compatibility of a binder and filler at the contact area is evaluated in terms of thermal dynamics [9—12]. It is known that biopolymers is used to strengthen soils [9, 11, 21—22].

The most common soils that need stabilizing are clay and loam. In order to stabilize them, in Russia various compounds are used, e.g., organic (Permazine, USA), Dorzon (Ukraine), ECOROAD (USA), lime (*Roadbond* (South Africa), *SuperMix* (Russia), acid (*RoadPaker Plus* (Canada), *RPP-235* (Germany), *CBR+* (South Africa), polymer emulsions (*LBS* (USA), *M10+50* (USA), *LDC+12* (USA), Nanostab (Germany), Dorstab (Russia), ECOLUX (Russia) [13, 16]. The objective of strengthening clay soils and alkalis is to improve their operational characteristics, i.e. to enhance their resistance to cracks and water, wear and tear [6, 8, 14]. A supplement be competitive compared to other ones already in use in road industry.

Considering all of the above, we have synthesized an organic and mineral modifier based on water polyvinylacetate dispersion containing both mineral and organic binders.

1. An organic and mineral modifier of cement soil was synthesized using the following components:

- polyvinylacetate dispersion (PVA) D7D 51/15 v, homopolymeric rough disperse according to the GOST (ГОСТ) 18992-80 with the mass concentration of a solid residual of 50—55 %;
- a liquid sodium glass according to the GOST (ГОСТ) 13078-81 with the mass concentration of silicon oxide of 27—30 %;
- carboxymethylcellulose.

The chemical properties of PVA are determined by complex ether groups and graft chains joined with the main chain with complex ether bonds. PVA is soaped with water solutions of acids and alkalis and undergo alcoholysis under the effect of catalytic amounts of acids and alcohols of alkali metals in waterless media with the formation of polyvinyl alcohol. The equation of the reaction is given in Fig. 1 [18].

Along with the main reactions there are side reactions of saponification of small amounts of nonpolymerized vinyl acetate and acetic-acid ether as shown in Fig. 2 [18].

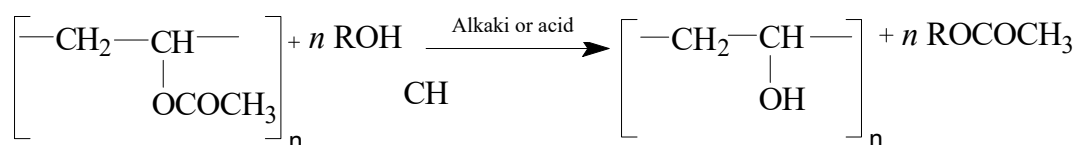


Fig. 1. Alcoholysis reaction of PVA

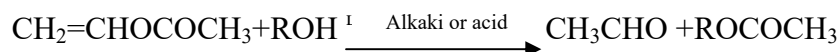


Fig. 2. Saponification of vinyl acetate

The carboxymethylcellulose is a poly-1,3-glycol. This structure is predominant, but there is always some small amount of additives by means of α -carbon atom which results in up to 2 % of hydroxyl groups in the carboxymethylcellulose being located in the same way as in 1,2-glycol. The formula of the carboxymethylcellulose is given in Fig. 3 [18].

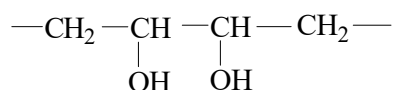


Fig. 3. Formula of the carboxymethylcellulose

The liquid sodium glass (alkaline solution of sodium silicates $\text{Na}_2\text{O} (\text{SiO}_2)_n$) during hydrolysis forms a silicic acid gel that is a construction glue, i.e. a binding component.

The silicic acid gel is formed according to the scheme shown in Fig. 4 [2, 13].

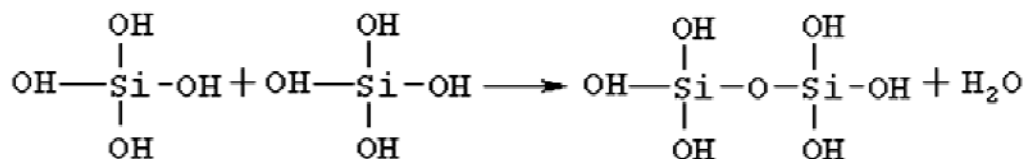


Fig. 4. Formation of the silicic acid gel

All the components of Portland cement clinker of the compound and phase interact with water and show hydration activity. As a result, there are hydrated cations-coagulants Ca^{2+} , Mg^{2+} , Al^{3+} , Fe^{3+} , etc. in the solution. They are absorbed by the surface of silica colloidal particles between the particles transforming them into aggregates. An approximate scheme of aggregation is shown in Fig. 5 [1, 2].



Fig. 5. Aggregation of silica colloidal particles

In the liquid glass there are hydrated ions-silicates and hydrosilicates that form a solidification system CaSiO_3 with free hydrated ions of Ca^{2+} of the cement water solution. A reduced ionic equation of the system is shown in Fig. 6 [15].

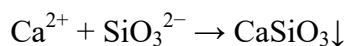


Fig. 6. Formation of water insoluble calcium silicate

We believe that polymerization of carboxymethylcellulose can also be accompanied with the formation of bridges through the calcium ions resulting in the structures exemplified in Fig. 7. According to the above theoretical concepts, a preliminary analysis of the original data was performed and the experiment was planned.

The synthesis was carried out with 1 litre of an organic and mineral additive. In a beaker per 1 litre 200 g of 46 % dispersion of PVA with $\text{pH} = 7$ was weighed. As it was heated up to 60—70 °C and constantly stirred, 500 ml of water was added followed by 25 ml of liquid sodium glass. pH was measured as 12. Mixing as the temperature was maintained took place over 7 days till complete homogenization. pH was found to drop to 9. Then 50 % solution of CaCl_2 (40 ml) was gradually introduced with pH being controlled.

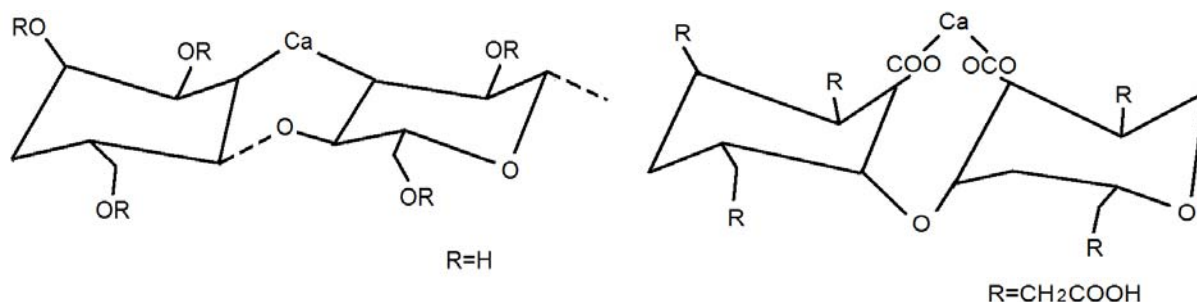


Fig. 7. Polymerization of carboxymethylcellulose

As pH dropped to 7, 100 g of 14 % of carboxymethylcellulose as a dispersion stabilizer was added. As a result we obtained 450 ml of organic and mineral modifier (24 % solution).

2. Study of the influence of synthesized organic and mineral modifier on the properties of cement soils. In order to make samples of non-modified and modified cement soils, Portland cement (PC) by the Stariy Oskol Cement Plant (CEM I according to the GOST (ГОСТ) 31108-2003), clayous soil and synthesized complex additive (CA) were used.

The samples were of a cylindrical form sized 3×3 cm according to the recipes presented in Table 1 and 2.

Table 1

Composition of cement samples

System	Mass concentration, %	Loading during formation, MPa	Mass of cement, g	Mass of CA, g	Mass of water, g
PC + water 10 %	0	10	37	—	3.7
PC + CA + water 10 %	0.05		37	0.075	3.6
	0.10		37	0.15	3.58
	0.50		37	0.77	3.1
	1.00		37	1.54	2.53
	2.00		37	3	1.44
PC + water 10 %	0	20	40	—	4
PC + CA + water 10 %	0.05		40	0.08	3.94
	0.10		40	0.16	3.88
	0.50		40	0.83	3.37
	1.00		40	1.66	2.74
	2.00		40	3.33	1.47

The analysis of the kinetics of an increase in the strength of the samples made under the load of 10 MPa found that the use of a complex additive in different concentrations reduces the strength of a modified cement rock compared to that in the samples by 1.5—2 times.

Hence after day 1 the strength of the samples was 31 MPa and that of the CA samples was 19, 20, 17, 15 and 14 MPa with 0.05, 0.1, 0.5, 1 and 2 % of the additive respectively.

After day 3 the strength of a cement rock went up two-fold except the samples with 0.1 % of CA. From day 3 to day 7 the solidification showed a significant slowdown in the samples with the compositions 2 and 6. By day 14 the strength of the samples with 0.05 and 0.1 % of CA dropped to 23 and 21 MPa respectively. By day 28 the strength of the samples was 63 MPa, while in those with the additive only by 30—39 MPa. The strongest out

of the modified samples were the samples with 1 % of the additive and the least strong ones with 0.1 %.

Table 2

Composition of cement soil samples

System	Mass concentration of CA, %	Loading during formation, MPa	Mass of soil, g	Mass of cement, g	Mass of CA, g	Mass of water, g
Soil + CA (10 %) + + water (17 % of the mass of CS)	0	10	33.5	3.35	—	6.26
Soil + PC (10 %) + + CA + water (17 % of the mass of CS)	0.05		33.5	3.35	0.08	6.10
	0.10		33.5	3.35	0.15	6.00
	0.50		33.5	3.35	0.75	5.60
	1.00		33.5	3.35	1.50	5.00
	2.00		33.5	3.35	3.00	4.00
Soil + PC (10 %) + + water (17 % of the mass of CS)	0	20	36.0	3.60	—	6.70
Soil + PC (10 %) + + CA + water (17 % of the mass of CS)	0.05		36.0	3.60	0.08	6.60
	0.10		36.0	3.60	0.16	6.58
	0.50		36.0	3.60	0.83	6.00
	1.00		36.0	3.60	1.66	5.40
	2.00		36.0	3.60	3.33	4.20

The formation was performed using a universal testing machine UMM-20 with an extra load of 10 and 20 MPa. The formed samples were stored in a desiccator at room temperature for 28 days. The compressive strength limit was determined after 1, 3, 7, 14 and 28 days according to the Russian standards and international practices [7, 25]. The kinetics of an increase in the strength of the samples from non-modified and modified soils is presented in Fig. 8—11.

The analysis of the kinetics of an increase in the strength of the samples made under a load of 20 MPa found that after 3 days the strength of the samples with the additive is 10 MPa as much as for the samples (35 MPa). By day 7 there was a drop in the strength of the samples containing 0.1 and 0.5 % of a complex additive. From day 7 to day 14 there was a sharp decrease in the strength from 48 to 21 MPa in the samples containing 0.1 % of CA. Then the strength was also on a sharp rise and after 28 days it was 45 MPa. The strength of the remaining samples was increasing with no changes in the speed. The strength of the samples on day 28 was 107 MPa.

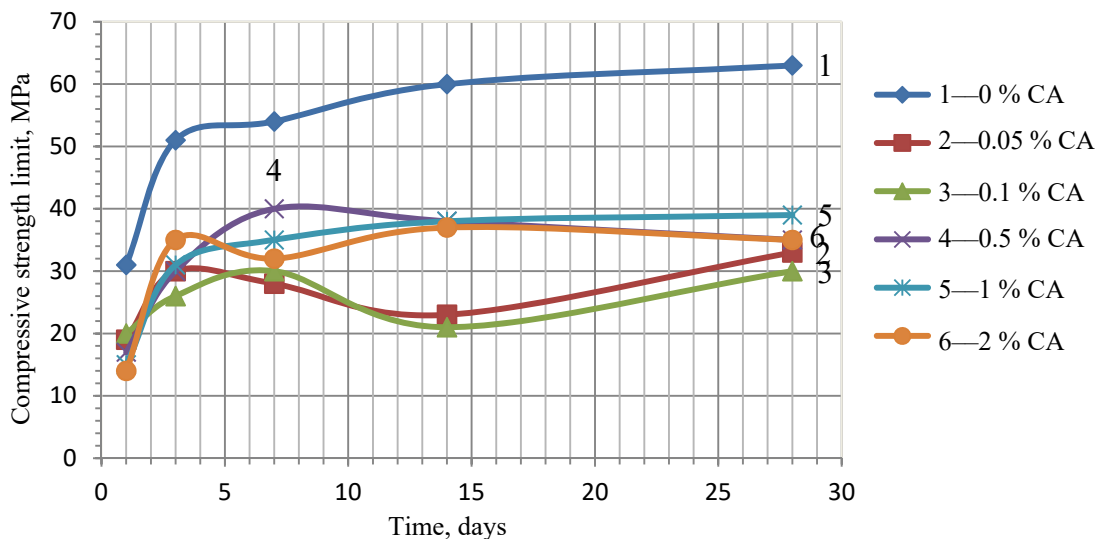


Fig. 8. Kinetics of an increase in the strength of the samples of a cement rock with the water content of 10 % formed under a load of 10 MPa

It was noted that the strength of the samples made at the pressure 20 MPa for both the samples as well as those containing 1.5 times as much CA as those made at 10 MPa.

The tests of the cement soil samples formed under the press pressure of 10 MPa found that an addition of CA has no effect on their daily strength while on day 3 the strength went up from 4.6 to 5 MPa in the samples with 0.05 and 0.1 % of CA.

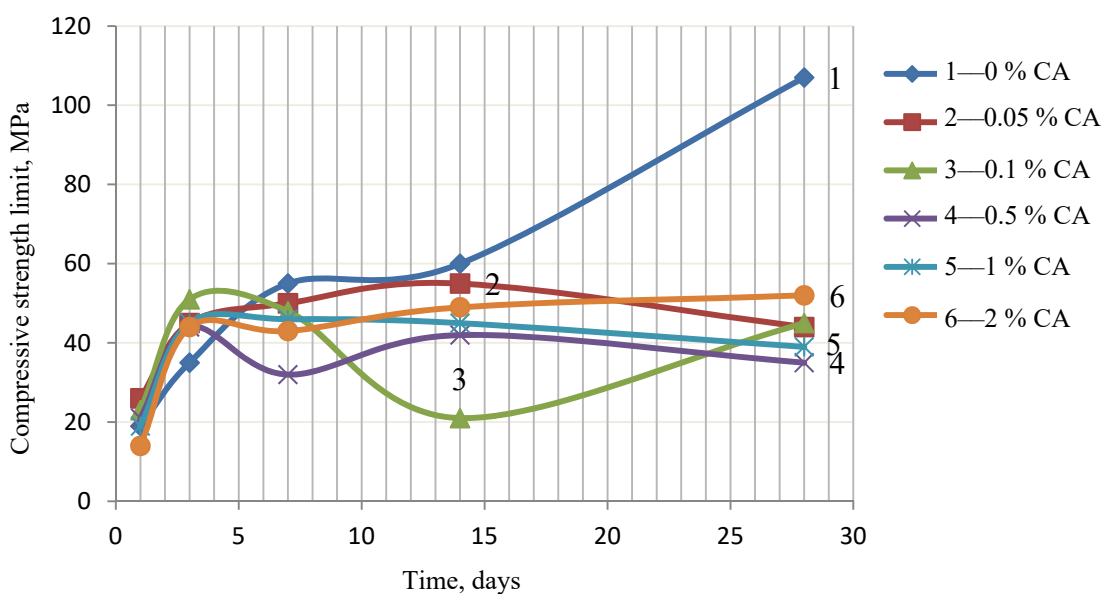


Fig. 9. Kinetics of an increase in the strength of the cement rock samples with the water content of 10 % formed under the load of 20 MPa

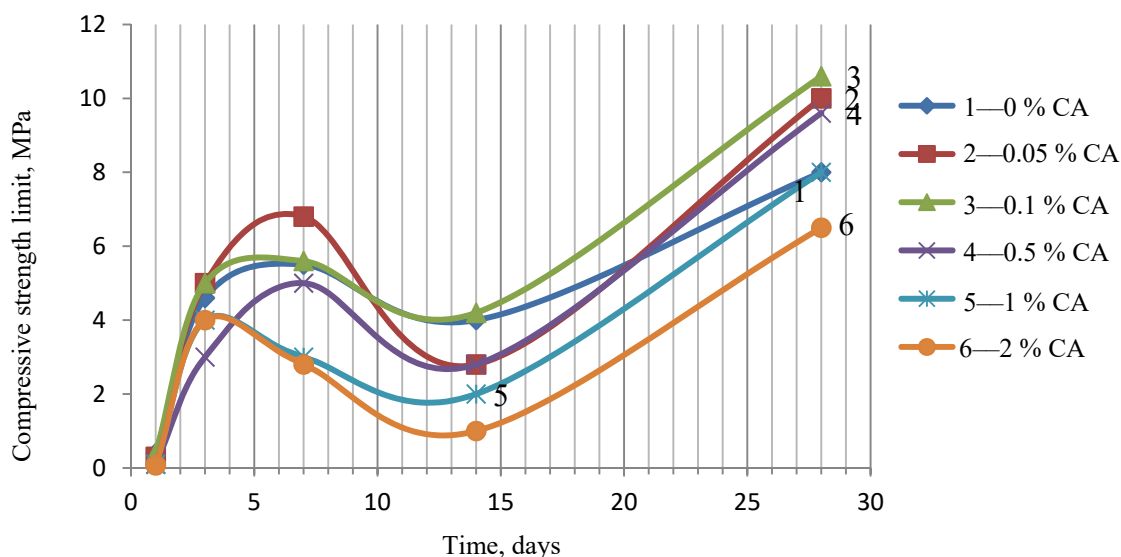


Fig. 10. Kinetics of an increase in the strength of the cement soil samples with the water content of 17 % and PC 10 % formed under the load of 10 MPa

From day 3 to day 7 solidification slowed down in the samples of the compositions 5 and 6, the strength of the remaining samples did not change. On day 14 there was a significant drop by 1.5—2 times in the strength of the samples of all the compositions as well as a sharp rise on day 28. Hence the strength of the samples was 10, 10.6, 9.6, 8 and 6.5 MPa with 0.05, 0.1, 0.5, 1 and 2 % of CA with the strength of the samples being 8 MPa.

A similar thing holds true for the tests of the samples formed at 20 MPa where the introduction of CA has no effect on the daily strength.

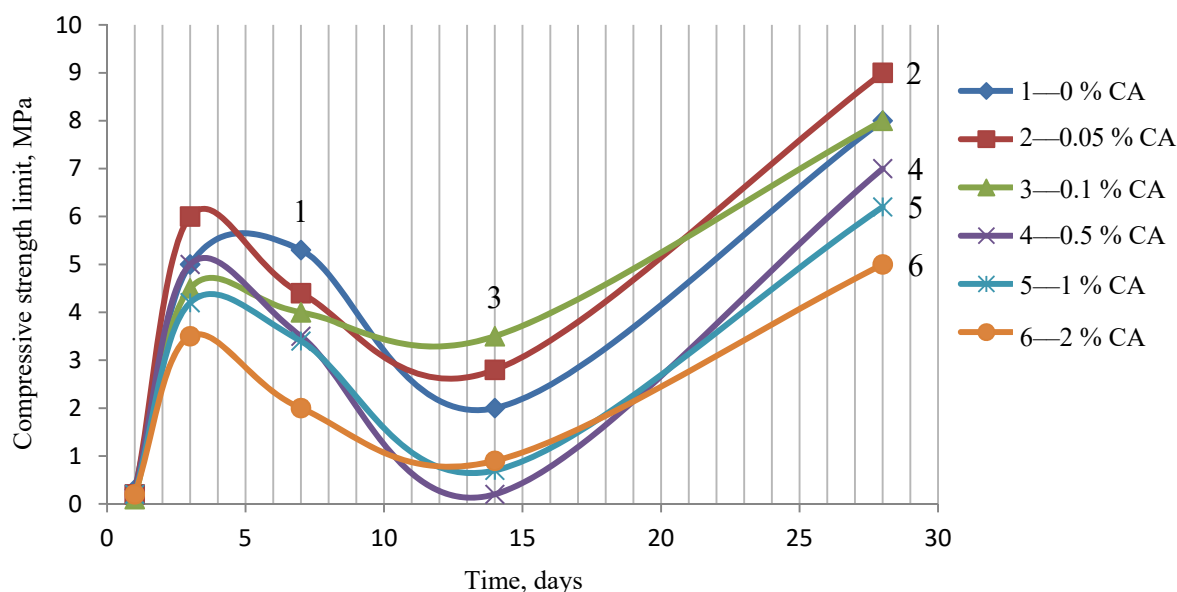


Fig. 11. Kinetics of an increase in the strength of the cement soil samples with the water content of 17 % and PC of 10 % formed under the laod of 20 MPa

However, on day 3 the strength of the samples with 0.05 % of the additive rose from 6 MPa at 5 MPa for the samples while the strength of the remaining samples was no more than 3 MPa. Then up to day 14 the strength of the samples of all the compositions saw a considerable decrease and was on a sharp rise by day 28. On day 28 the strength of the samples with 0.05, 0.5, 1 and 2 % of CA was 9, 7, 6.2 and 5 MPa respectively at the strength of the samples of no more than 8 MPa.

Summarizing the results of the experiment, note that high concentrations of an organic and mineral additive in the mixing water contributed to a slow down in the setting, solidification and reduced the strength of the cement rock while in low concentrations, particularly 0.05—0.1 %, there is an increase in the strength of cement soil, which correlates with the data presented by the authors in [7].

Conclusions. The comparison of the kinetic curves of solidification of the cement rock samples formed under the pressure of 10 MPa revealed that during the formation of a crystallizing structure the strength drops due to an amorphous group of polymers and dominance of coagulation structures:

- a) at the stage of the formation of fetuses of a new phase, agglomeration of colloidal particles;
- b) at the stage of the formation of a spatial coagulation structure, coalescence of emerging crystals;
- c) at the stage of the formation of an entire carcass of a crystallizing structure;
- d) at the stage of the growth of the crystals and formation of the contacts of intersection and intergrowth;
- e) at the stage of “aging” of hydrate new formations, i.e. crystallization and recrystallization of metastable crystals of hydro-sulphate-aluminates of calcium.

CA increases the strength of the cement rock formed at the pressure of 20 MPa, at the 2nd—3rd stage of the solidification due to a decrease in the thickness of a layer of the polymer component and an increase in the number of contact and condensation and crystal bonds [7].

The comparison of the kinetic curves of solidification of the cement soil samples formed at the pressure of 10 and 20 MPa revealed that the introduction of 0.05—0.1 % of CA provides growth in the strength of the solidification systems by almost 1.5 times due to structural and functional groups of a complex modifier with active centres of clayous soils.

The improvement of the physical and mechanical properties of the indices is indicated by the capacity of CA to fill a capillary structure of clayous soils with subsequent structure formation due to Ca^{2+} ions and polar groups of clay micelles, a polymer part of CA according to the

above schemes. A practical importance is determined by the capacity of a modifier to enhance the strength and operational functions of temporary entry ways and long areas of highways designed using challenging clayous soils.

References

1. Artamonova O. V., Kukina O. B., Solokhin M. A. [Study of the kinetics of hydration and strength of cement stone modified by complex nano-additive]. *Trudy V Mezhdunarodnoi konferentsii «Deformatsiya i razrushenie materialov i nanomaterialov (DFMN—13)»* [Proc. of the V International conference «Deformation and destruction of materials and nanomaterials (DFMN-13)»]. Moscow, 2013, pp. 638—640.
2. Artamonova O. V., Kukina O. B., Solokhin M. A. Issledovanie struktury i svoystv tsementnogo kamnya, modifitsirovan-nogo kompleksnoi nanodobavkoi [Study of the structure and properties of cement stone modified by complex nano-additive]. *Deformatsiya i raz-rushenie materialov*, 2014, no. 11, pp. 18—22.
3. Bondar' I. S. Sdvigovye ispytaniya svyaznykh gruntov pri razlichnykh traektoriyakh nagruzheniya [Shear testing of cohesive soils under the various loading trajectories]. *Inzhenerno-stroitel'nyi zhurnal*, 2012, no. 7, pp. 50—57.
4. Vatin N. I., Petrosov D. V., Kalachev A. I., Lakhtinen P. Primenenie zol i zoloshlakovykh otkhodov v stroitel'stve [Application of ash and ash waste in construction]. *Inzhenerno-stroitel'nyi zhurnal*, 2011, no. 4, pp. 16—20.
5. Vdovin E. A., Mavliev L. F., Bulanov P. E. Vzaimodeistvie kompleksnoi dobavki na osnove oktiltrietoksisilana i gid-roksida natriya s osnovnymi komponentami grunta dorozhnogo naznacheniya [Interaction of a complex additive based on octyltriethoxysilane and sodium hydroxide with the main components of the road soil]. *Izvestiya KGASU*, 2015, no. 1 (31), pp. 165—170.
6. Vdovin E. A., Mavliev L. F. Issledovanie dolgovechnosti modifitsirovannogo tsementogrunta dorozhnogo naznacheniya [Study of durability of modified cementownia road destination]. *Promyshlennoe i grazhdanskoe stroitel'stvo*, 2014, no. 11, pp. 76—79.
7. Vdovin E. A., Mavliev L. F. Povyshenie kachestva ukreplennykh gruntov vvedeniem gidrofobiziruyushchikh dobavok [Improvement of the quality of fortified soils by the introduction of hydrophobic additives]. *Izvestiya KazGASU*, 2012, no. 4 (22), pp. 373—377.
8. Vdovin E. A., Stroganov V. F., Mavliev L. F. Puti povysheniya effektivnosti ukrepleniya gruntov dlya stroitel'stva dorozhnykh odezhd [Ways to improve the efficiency of strengthening the soil for the construction of road pavement]. *Vestnik SibADI*, 2013, no. 1 (29), pp. 52—58.
9. Glazkov, S. S. Kriterii termodinamicheskoi ustoichivosti polimernykh i kompozitsionnykh materialov [Criteria of thermodynamic stability of polymeric and composite materials]. *Stroitel'nye materialy*, 2007, no. 1, pp. 63—65.
10. Glazkov S. S., Kozlov V. A. Model'noe rassmotrenie uslovii sovmestimosti v kompozitsionnoi sisteme pri kontakte dvukh faz [Model study of the conditions of compatibility in the composite system upon contact of the two phases]. *Izvestiya vuzov. Stroitel'stvo*, 2008, no. 9, pp. 99—105.
11. Glazkov S. S., Kukina O. B., Budasov S. B., Cherepakhin A. M. Razrabotka kompleksnoi stabiliziruyushchei dobavki dlya tsementogruntoy [Development of the complex-stabilizing additives for seminterrato]. *Nauchnyi*

vestnik Voronezhskogo GASU. Fiziko-khimicheskie problemy i vysokie tekhnologii stroitel'nogo materialovedeniya, 2014, no. 2 (9), pp. 53—58.

12. Glazkov S. S., Snycheva E. V., Rudakov O. B. Raschet stepeni sovmestimosti napolnitelya i svyazuyushchego v kompozitsionnykh materialakh [Calculation of the degree of compatibility of filler and binder in composite materials]. *Izvestiya vuzov. Stroitel'stvo*, 2006, no. 6, pp. 100—103.

13. Zagorodnykh K. S., Kukina O. B., Glazkov S. S., Cherepakhin A. M. Issledovanie vozmozhnosti primeneniya kompleksnoi dobavki k tseментu pri stabilizatsii gruntov [Study of the possibility of using a complex additive to cement in soil stabilization]. *Nauchnyi vestnik Voronezhskogo GASU. Fiziko-khimicheskie problemy i vysokie tekhnologii stroitel'nogo materialovedeniya*, 2016, no. 1 (12), pp. 20—24.

14. Zolotukhin S. N., Kukina O. B., Abramenko A. A., Savenkova E. A., Solov'eva E. A., Novikova K. K. Bestsementnye bezobzhigovye stroitel'nye materialy s ispol'zovaniem fosfogipsa [Cementless non-ignition building materials using phosphogypsum]. *Nauchnyi vestnik Voronezhskogo GASU. Vysokie tekhnologii. Ekologiya*, 2016, no. 1, pp. 115—121.

15. Kukina O. B. *Tekhnogennyye karbonatkal'tsievye otkhody i tekhnologiya ikh ispol'zovaniya v stroitel'nykh materialakh s uchetom strukturoobrazuyushchei roli*. Diss. kand. tekhn. nauk [Technogenic calcium carbonate waste and technology of their use in construction materials that incorporate structure-forming role. Cand. eng. sci. diss.]. Voronezh, 2002. 184 p.

16. Lundyshev I. A. Kompleksnoe primeneniye monolitnogo penobetona pri stroitel'stve v trudnodostupnykh raionakh dobychi energoresursov [Complex application of monolithic foam concrete in construction in hard-to-reach areas of energy production]. *Inzhenerno-stroitel'nyi zhurnal*, 2009, no. 4, pp. 16—20.

17. L'vova O. M., Tulin P. K. Podzemnye avtomatizirovannyye parkingi v tsentre goroda [Underground automated parkings in the centre of the city]. *Inzhenerno-stroitel'nyi zhurnal*, 2009, no. 4, pp. 11—15.

18. Rozenberg M. E. *Polimery na osnove vinilatsetata* [Polymers of vinyl acetate]. Leningrad, Khimiya Publ., 1983, 176 p.

19. Ustyan N. A. Geokonteynery v dorozhnom i gidrotekhnicheskome stroitel'stve [Geocontainers in road and hydraulic engineering construction]. *Inzhenerno-stroitel'nyi zhurnal*, 2011, no. 4, pp. 22—25.

20. Chan C.-M. Strength and stiffness of a cement-stabilised lateritic soil with granulated rubber addition Proceedings of the Institution of Civil Engineers. Ground Improvement, 2012, no. 165 (1), pp. 41—52.

21. Chang I., Im J., Prasadhi A. K., Cho G.-C. Effects of Xanthan gum biopolymer on soil strengthening. Construction and Building Materials, 2015, no. 74, pp. 65—72.

22. Chang I., Im J., Prasadhi A. K., Cho G.-C. Soil strengthening using thermo-gelation biopolymers. Construction and Building Materials, 2015, no. 77, pp. 430—438.

23. Filipiak J. Fly ash in construction industry. Strength tests of soil stabilized with mixture of ash and cement [Popiół lotny w budownictwie. Badania wytrzymałościowe gruntów stabilizowanych mieszanką popiołowo-cementową]. *Rocznik Ochrona Srodowiska*, 2011, no. 13 (1), pp. 1043—1054.

24. Im J., Chang I., Cho G.-C. Small strain stiffness and elastic behavior of gellan treated soils with confinement. Geotechnical Special Publication (GSP 280), 2017, pp. 834—841.

25. Wells D., Egan J. A., Murphy D. G., Paret T. Ground shaking and structural response of the Washington monument during the 2011 mineral, virginia, earthquake. Special Paper of the Geological Society of America, 2014, no. 509, pp. 199—233.